## END TERM EXAMINATION

FIFTH SEMESTER [B.TECH] DECEMBER 2015-JANUARY 2016

Paper Code: ETCE-307

Subject: Engineering Hydrology

Time: 3 Hours

Maximum Marks: 75

Note: Attempt any five questions including Q.no.1 which is compulsory.

Select one question from each unit.

Answer any five questions providing sketches wherever applicable:(5x5=25)

Describe the hydrologic cycle, and explain the water budget equation.

(b) Define Evapotranspiration and name different methods of estimating evapotranspiration of a catchment.

Describe Slope-Area method for estimating discharge through a

(d) What are the factors affecting a flood hydrograph? Define a Unit Hydrograph.

(e) Starting with the continuity equation, describe the modified Pul's method of reservoir routing.

- (f) Why is separation of baseflow required from a flood hydrograph?

  Describe three methods of baseflow separation.
- (g) Derive an expression for steady-state radial flow into a well fully penetrating an unconfined aquifer.

## Unit-I

- Q2 (a) Describe either the Thiessen polygon or the Isohyetal method of estimating the mean areal rainfall over a catchment. Write an expression for determining the optimum number of rain gauge stations in a catchment for a specified allowable degree of error. (4+1)
  - (b) On the basis of isopluvial maps, the 50 year-24 hour maximum rainfall at a place is found to be 17 cm. Determine the probability of a 24 hour rainfall of magnitude equal to or greater than 17 cm occurring at that place (i) once in 25 successive years. (2+2)

(c) Describe the Intensity-Duration-Frequency relationship of storms over a catchment providing a general form of the relationship. Define 'Return Period' of a hydrological event.

- Q3 (a) Write Horton's expression of decay of infiltration capacity with time explaining each term of the expression. Hence, find the infiltration capacity of a catchment (i) at the 6th hour (hr) from the start and (ii) the total volume of infiltration over the 6 hour period if the initial rate of infiltration is 5.3 hr<sup>-1</sup>, the final capacity is 0.5 cm hr<sup>-1</sup>, and the time constant is 0.4 hr<sup>-1</sup>.
  - (b) Define Φ-index. For a 12-hour (hr) storm rainfall over a catchment with the hourly distribution of rainfall in cm given by 2.0, 2.5, 7.6, 3.8, 10.6, 5.0, 7.0, 10.0, 6.4, 3.8, 1.4 and 1.4 and estimated direct runoff 25.5 cm, show that the average Φ-index for the catchment would be 3.5875 cm hr<sup>-1</sup>.

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(a) What is the usefulness of measuring the state at a gauging site? Describe any one type of automatic stage recorder by providing a

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(b) What are the different types of current meter? Describe different procedure of measuring the velocity of a stream using a current

(c) From the following data for measuring discharge of a river at a Sauging site, estimate the discharge by calculating by the mid-section

Distance from the Velocity (ms-1) Depth of flow (m) left water edge (m) 0.0 0 1.1 2.0 2.5 2.0 0.229 0.326 0.411 0.336 ω S 0.260 0.183 1

(4.5)

(a) What are the uses of a flow duration curve and a flow-mass curve in water resource planning and development?

Q5

(b) Estimate the storage required in a proposed reservoir to meet variable on the downstream of the reservoir and the net evaporation (Mm<sup>3</sup>). after according for rainfall over the reservoir's area. The table also includes data of mandatory release (Mm<sup>3</sup>) to be made monthly demands in Million Cubic Metre (Mm³) as tabulated below.

	Evaporation	Release	Mandatory	Demand	Inflow	Month
	2		11	60	50	c,
	3		9	80	40	ŦJ
	3		11	80	30	Z
	4		10	90	25	A
	4		11	120	30	X
and the contract of the contra	2		10	130	40	J
	0		11	40	200	٦
	-		=	30	250	Α
	1		10	50	175	S
	ω		=	50	100	0
	2		10	50	70	z
	ω		11	50	60	ם

(a) What are the assumption made in the Unit Hydrograph (UH) theory a UH is available. case of the required duration being fraction of the duration for which duration of a UH that is already available. Describe the method in the Name the methods of deriving a new UH of duration different form the (2.5+2+3)

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(b) The observe flow in m<sup>3</sup>s<sup>-1</sup> from a storm of 6-hour duration at a stream gadging site having catchment area of 500 km<sup>2</sup> are given below. Assuming the base flow to be negligible, derive the ordinates of a 6-hour unit hydrograph.

(	<ul> <li>(a) Describe a procedure for deriving UH from a complex storm.</li> <li>(b) Regression equations of a Synthetic Unit Hydrograph (SUH) of 1-day duration for hydrometeorologically homogeneous subzone 2(a) of India as given in the Flood Estimation Report of the Central Water Communication are reproduced below. Evaluate the parameters of the P.T.O.</li> </ul>	Time (hr)         0         6         12         18         24         30         36         42         48         54         60         66         72           Flow         0         100         250         200         150         100         70         50         35         25         15         5         0	
	(complex sydrograph (ydrograph leous subz rt of the uate the pa	42 48 50 35	
	storm (SU) one (Cen	25	1
	H) of 2(a) of trail	15	1
	(4) f 1-day of India Water s of the P.T.O.	5 6	
	o he ter	0 72	

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(a) Write the general equation of hydrologic frequency analysis? Describe procedure for estimating the design flood of a specified return period for practical application using Gumbel's method?

Estimate the peak flood with a return period of 300 years.

examples.

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(b) A 10 cm diameter well penetrates a 10 m thick confined aquifer. The steady state drawdowns were found to be 2.5 m and 0.05 m at distances of 10 m and 40 m respectively from the centre of the wall when the well was pumped at a constant rate of 125 litre/min. Calculate the transmissibility and hydraulic conductivity of the

(c) What purposes does artificial recharge serve? Describe any one method of artificial recharge by providing a sketch

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duration is 2.2 cm, what would be the peak flow of the resulting direct runoff hydrograph at the weir site? application? If the depth of effective rainfall of a design storm of 1-day Sketch the resulting SUH. How is such an SUH finalized for practical SUH at a weir site on a river in that subzone using relevant catchment parameters provide below, and by following the usual convention.

Qp (hour)	T <sub>m</sub> (hour)	T <sub>B</sub> (hour)	WR75 (hour)	Wese (hour)	W75 (hour)	W <sub>50</sub> (hour)	q <sub>p</sub> (m <sup>3</sup> s <sup>-1</sup> )	t <sub>e</sub> (hour)	SUH Parameters
Q <sub>p</sub> A	$t_0 + t_1/2$	5.428 (tp)0.852	0.440 (qp)-0.918	0.856 (qp)-0.865	1.028 (q <sub>p</sub> )-1.071	2.084 (qp)-1.065	2.164 (qp)-0.940	2.272 (LLc/S)-0.409	Regression Equation

Stream slope 'S' (m/km)	CG to dam site 'Le' (km)	Length of the stream from	L' (km)	Length of longest stream	Area 'A' (km²)	Catchment parameter
64.6		10.05		18.19	120	Value

(b) Pcak floods of return periods 50 and 100 year in a river were (a) Define with the help of suitable sketches: (i) an aquifer, (ii) an aquitard, (CLDefine Standard Project Flood (SPF) and Probable Maximum Flood estimated as being 40809 m<sup>3</sup>s<sup>-1</sup> and 46300 m<sup>3</sup>s<sup>-1</sup> respectively (iii) an aquiclude, (iv) an aquifuge, and (v) a leaky aquifer giving 1 (4.5)

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